

Cornish (1907)

Early photograph of a train of roll waves in the Swiss Alps.

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## THE STATES OF FLOW

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**ABSTRACT.** Four dimensionless numbers in open-channel flow are presented, explained and compared. Two of them are ratios of velocities, and the other two are ratios of diffusivities. All four numbers are defined in terms of veritable physical quantities, either velocities or diffusivities. Taken together, these numbers complete the description of the state of flow, for either steady flow (the first two numbers), or unsteady flow (the last two).

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### 1. INTRODUCTION

There are two characteristic properties in open-channel flow: (1) velocity; and (2) diffusivity.

The velocity of a fluid parcel is the rate of change of its position in space, in a particular direction, with time. The units of velocity are  $L/T$ , in which  $L$  = length, and  $T$  = time. The expression  $u = 1 \text{ m/sec}$  (one meter per second) describes the mathematical certainty that a chosen fluid parcel is moving along an established flow direction, or path, with a velocity  $u$  equal to 1 meter per second. In fluid mechanics, velocity relates to the process of *convection* of a fluid parcel; in hydrology, it relates to *concentration*, a concept related to *time of concentration*. In hydraulic and hydrologic modeling, velocity is described by a differential equation of **first order**.

Surface flow diffusivity  $\nu$  is the *first moment* of the flow velocity. The units of diffusivity are  $(L/T)L$ , or its equivalent  $L^2/T$ . The expression  $\nu = 1 \text{ m}^2/\text{sec}$ , in relation to a given disturbance, describes the mathematical certainty that the disturbance is spreading at the rate controlled by the coefficient of diffusivity  $\nu$ . In fluid mechanics, diffusivity relates to the process of diffusion; in hydrology, it relates to flood wave *attenuation*, or *dissipation*. In hydraulic and hydrologic modeling, diffusivity is described by a differential equation of **second order** (Table 1).

Table 1. Velocities and diffusivities in open-channel flow.				
Property	Symbol	Units	Process	Order
Velocity	$u$	$L/T$	Convection	First
Diffusivity	$\nu$	$L^2/T$	Diffusion	Second

These two fluid properties, velocity and diffusivity, characterize the flow up to second order. Several types of velocities and diffusivities may be construed, with their ratios constituting the dimensionless parameters referred to as "numbers." The latter encapsulate the properties of fluid flow, enhancing their understanding under both steady and unsteady conditions. This fact is reflected in the title of the present article: *The states of flow*, referring to the various states under which the flow may be described using these numbers. The remainder of this article endeavors to explain the numbers, clarifying their definition and scope.

## 2. VELOCITIES IN OPEN-CHANNEL FLOW

There are three characteristic velocities in open-channel flow: (1) the mean velocity of steady flow  $u$ ; (2) the relative celerity of kinematic waves  $v$ ; and (3) the relative celerity of dynamic waves  $w$ . Celerity is the velocity of a wave (unsteady flow), as opposed to the velocity of steady flow (Ponce, 1991).

The celerity of a kinematic wave is:  $c_k = \beta u$ , in which  $\beta$  = exponent of the rating,  $Q = \alpha A^\beta$ , in which  $Q$  = discharge,  $A$  = flow area, and  $\alpha$  = coefficient of the rating. Therefore, the relative celerity of a kinematic wave is:  $v = c_k - u$ , i.e., the velocity of the kinematic wave relative to that of the flow (Ponce, 2014a).

The celerity of a dynamic wave, which has two components, is:  $c_d = u \pm (gD)^{1/2}$ , in which

$g$  = gravitational acceleration,  $D$  = hydraulic depth, with  $D = A / T$ , and  $T$  = channel (stream) top width. Therefore, the relative celerity of a dynamic wave is:  $w = c_d - u = \pm (gD)^{1/2}$ , i.e., the velocity of a dynamic wave relative to that of the flow (**Ponce, 2014b**).

The three velocities identified here encompass both steady ( $u$ ) and unsteady flow ( $v$  and  $w$ ), as well as short waves (dynamic,  $w$ ), and long waves (kinematic,  $v$ ). We note that these are the only flow velocities that are identifiable in the present context.

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### 3. DIFFUSIVITIES IN OPEN-CHANNEL FLOW

Three diffusivities are recognized in open-channel flow: (1) molecular diffusivity; (2) hydraulic diffusivity; and (3) *spectral* diffusivity. In fluid mechanics, the molecular diffusivity  $\nu_m$  is commonly referred to as *kinematic viscosity*  $\nu$ , a measure of the fluid's internal resistance to flow at the molecular level. In open-channel flow, the hydraulic diffusivity is expressed in terms of the bottom slope and bottom friction. In unsteady open-channel flow, the spectral diffusivity is defined in terms of the wavelength of the sinusoidal perturbation to the steady flow. These propositions are explained in **Box A**.

#### Box A. Diffusivities in open-channel flow.

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1. Newton's law of viscosity is:  $\tau / \rho = \nu (\partial u / \partial s)$ , in which  $\tau$  = shear stress,  $\rho$  = mass density of the fluid,  $\nu$  = kinematic viscosity of the fluid, and  $(\partial u / \partial s)$  = velocity gradient in the direction  $s$  perpendicular to the direction of  $\tau$ .
2.  $\tau / \rho = \nu_m (\partial u / \partial s)$ , in which  $\nu_m$  = molecular diffusivity.
3. The **molecular diffusivity**  $\nu_m$  may be expressed as  $\nu_m = u (L_m / 2)$ , in which  $L_m = (2\nu_m / u)$  is a characteristic *molecular* length (Chow, 1959).
4. The **hydraulic diffusivity**  $\nu_h$  is defined as  $\nu_h = u (L_o / 2)$ , in which  $L_o = (d_o / S_o)$  is a characteristic hydraulic (reach) length, defined as the distance along the channel wherein the flow drops an elevation equal to its equilibrium depth (**Hayami, 1951; Ponce and Simons, 1977**).
5. The **spectral diffusivity**  $\nu_s$  is defined as  $\nu_s = u (L / 2)$ , in which  $L$  = characteristic wavelength of the sinusoidal surface perturbation (**Ponce, 1979**).
6. Note that all three diffusivities: (1) molecular, (2) hydraulic, and (3) spectral, are defined in terms of their respective characteristic lengths: (1) molecular length, (2) hydraulic (reach) length, and (3) spectral wavelength of the sinusoidal perturbation. Furthermore, note that the three diffusivities share a similar structure: A product of the convective velocity times one-half of a respective characteristic length.

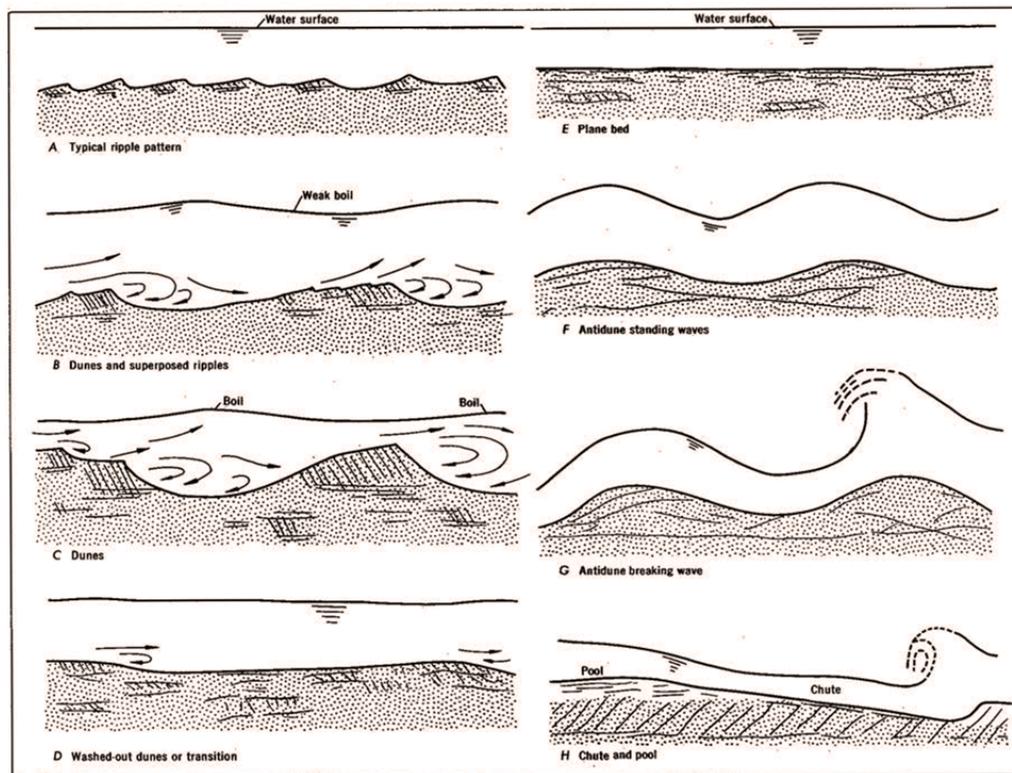
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### 4. STEADY FLOW VELOCITY RATIO: THE FROUDE NUMBER

The Froude number is the ratio of mean flow velocity  $u$  to relative celerity of dynamic waves  $w$ :  $F = u/w$  (Ponce, 2014b). It compares the mean flow velocity to the relative celerity of small (short) surface perturbations; accordingly, it classifies the flow into three types: (1) subcritical, for  $F < 1$ ; (2) critical, for  $F = 1$ ; and supercritical, for  $F > 1$ .

In open-channel hydraulics, the Froude number is useful in determining the direction of computation in the analysis of water surface profiles: Upstream for subcritical flow, and downstream for supercritical flow. In river mechanics, the Froude number is an indication of the limit between *lower regime*,  $F < 0.5$ , generally where ripples and dunes predominate in the streambed, and *upper regime*,  $F > 0.5$ , where plane bed and antidunes prevail (Simons and Richardson, 1966).

[Click on top of figure to expand]



Simons and Richardson (1966).

Fig. 1 Forms of bed roughness in alluvial channels: (a) lower regime (left), and (b) upper regime (right).

Note the sharp division between subcritical and supercritical flow at  $F = 1$ . The latter constitutes a singular point, at which the direction of computation switches instantaneously between upstream and downstream. The existence of a singularity in the neighborhood of  $F = 1$  may render the computation unstable; therefore, caution is recommended in the vicinity of critical flow. A history of the Froude number, including Froude's significant contributions to hydraulic engineering, has been presented by Ponce (2014b).

## 5. STEADY FLOW DIFFUSIVITY RATIO: THE REYNOLDS NUMBER

The Reynolds number is a ratio of viscosities, or diffusivities. The conventional Reynolds number, defined for an arbitrary cross-sectional shape, in terms of hydraulic radius  $R_o$ , is:  $R = (u_o R_o) / \nu_m$  (Ponce, 2014b). For a hydraulically wide channel:  $R = (u_o d_o) / \nu_m$ . For the purposes of this article, we

define an alternative Reynolds number as an appropriate ratio of diffusivities, as follows:  $\mathbf{R}' = \nu_h/\nu_m$ . Therefore:  $\mathbf{R}' = \mathbf{R} / (2 S_o)$  (see Table 2).

The Reynolds number  $\mathbf{R}$  classifies the flow as being in one of the following *regimes*: (1) laminar, (2) transitional, or (3) turbulent. In open-channel flow, under steady flow conditions, laminar flow occurs for  $\mathbf{R} \leq 500$ ; turbulent flow for  $\mathbf{R} > 2000$ ; and transitional flow in the intermediate range ( $500 < \mathbf{R} \leq 2000$ ). The use of the Reynolds number is somewhat limited in open-channel flow applications, since the flow usually remains within the turbulent regime.

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## 6. UNSTEADY FLOW VELOCITY RATIO: THE VEDERNIKOV NUMBER

The Vedernikov number is the ratio of the relative celerity of a kinematic wave  $v$  to the relative celerity of a dynamic wave  $w$ :  $\mathbf{V} = v/w$  (Ponce, 2014b). It characterizes the following states of flow:

- $\mathbf{V} < 1$ : Stable flow, for  $v < w$ ,
- $\mathbf{V} = 1$ : Neutrally stable flow, for  $v = w$ ,
- $\mathbf{V} > 1$ : Unstable flow, for  $v > w$ .

Under **stable** flow, the relative kinematic wave celerity  $v$  is smaller than the relative dynamic wave celerity  $w$  and, therefore, surface waves (perturbations) are able to attenuate (dissipate).

Under **neutrally stable** flow, the relative kinematic wave celerity  $v$  is *equal* to the relative dynamic wave celerity  $w$  and, therefore, surface waves neither attenuate nor amplify.

Under **unstable** flow, the relative kinematic wave celerity  $v$  is greater than the relative dynamic wave celerity  $w$ . Therefore, surface waves are subject to negative attenuation, i.e., amplification. In practice, the condition  $\mathbf{V} > 1$  leads to the development of *roll waves*, recognized as a train of waves that travel downstream, typically in artificial channels of steep slope (Fig. 2). The flow condition leading to the formation of roll waves may be explained in terms of the kinematic waves, which transport mass, overcoming the dynamic waves, which transport energy (Craya, 1952; Ponce and Choque Guzman, 2019).



Fig. 2 A train of roll waves in a lateral canal, Cabana-Mañazo irrigation, Puno, Peru.

The theory of the Vedernikov number (Vedernikov, 1945; 1946), originally christened by **Powell (1948)** and later presented by **Chow (1959)** in Chapter 8 of his textbook, was clarified by **Ponce (1991)**, who expressed the Vedernikov number in terms of relative kinematic and dynamic wave celerities. The subject of hydrodynamic stability for the control of roll waves in channelized rivers has been treated by **Ponce and Choque Guzman (2019)**.

## 7. UNSTEADY FLOW DIFFUSIVITY RATIO: DIMENSIONLESS WAVENUMBER

The dimensionless wavenumber of **Ponce and Simons (1977)** is defined as follows:  $\sigma_* = (2\pi/L)L_0$ . It may also be readily expressed as a ratio of diffusivities:  $\sigma_* = (2\pi/L)L_0 = 2\pi (v_h/v_s)$ . The dimensionless wavenumber  $\sigma_*$  classifies the unsteady flow being considered into four *spectral* ranges (Fig. 3):

1. Kinematic (extreme left),
2. Diffusion (left-of-center),
3. Mixed kinematic-dynamic (right-of-center), and
4. Dynamic (extreme right).

The precise domains of these spectral ranges have been determined by **Ponce (2023)**:

- Kinematic flow:  $\sigma_* < 0.001$ .
- Diffusion flow:  $0.001 \leq \sigma_* < 0.17$ .
- Mixed kinematic-dynamic flow:  $0.17 \leq \sigma_* < 1$  to 100, depending on the Froude number (refer to Fig. 3).
- Dynamic flow:  $\sigma_* \geq 10$  to 1000, depending on the Froude number (refer to Fig. 3).

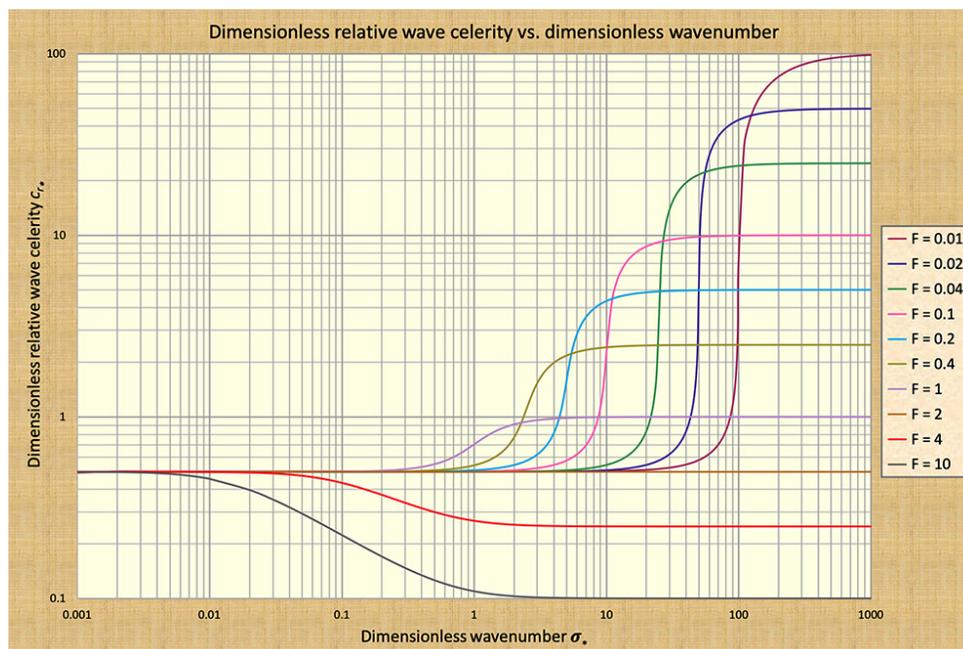


Fig. 3 Dimensionless relative wave celerity  $c_{f*}$  vs dimensionless wavenumber  $\sigma_*$ .

The findings of Ponce and Simons (1977) help to elucidate the behavior of *all* possible wave types in unsteady open-channel flow. The latter include both "long" waves, ostensibly of a kinematic nature, on the far left side of Fig. 3, and "short" waves, of a dynamic nature, on the far right. Also included are the diffusion waves, of intermediate range and displaying properties that are shown to be quite practical, and the mixed kinematic-dynamic waves, for short mixed waves, in the middle-to-right range. These mixed waves are, for the most part, impractical due to their extremely strong dissipative tendencies (Ponce, 2023).

## 8. SUMMARY

Four dimensionless numbers in open-channel flow are presented, explained and compared (Table 2). Two of them are ratios of velocities and the other two are ratios of diffusivities. All four numbers are defined in terms of veritable physical quantities, be it either velocities or diffusivities. Taken together, these numbers complete the description of the state of flow, for either steady flow (the first two numbers), or unsteady flow (the last two).

Dimensionless No.	Symbol	Ratio of	Definition	Ranges	Alternate
<i>Froude</i>	<b>F</b>	Velocities	$u / w$	(a) subcritical, (b) critical, (c) supercritical	None
<i>Reynolds</i>	<b>R'</b>	Diffusivities	$\nu_h / \nu_m$	(a) laminar, (b) transitional, (c) turbulent	<b>R</b>
<i>Vedernikov</i>	<b>V</b>	Velocities	$v / w$	(a) stable, (b) neutral, (c) unstable	None
<i>Ponce-Simons</i>	$\sigma_*$	Diffusivities	$2\pi (\nu_h / \nu_s)$	(a) kinematic, (b) mixed, (c) dynamic	None

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